



Department of Geography and Earth Sciences

**WRM 625**

Hydrological Modeling

Lecture 6

# Convex Routing Method

- Simplified procedure for routing hydrographs through stream reaches based on:

$$I - O = \frac{dS}{dt} \approx \frac{\Delta S}{\Delta t}$$

$$S_0 = b \left[ \left( \frac{O}{a} \right)^{1/d} \right]^m = b \left( \frac{O}{a} \right)^{\frac{m}{d}} = b \left[ \left( \frac{1}{a} \right)^{\frac{m}{d}} \right] O^{m/d} = K O^{m/d}$$

$$S = xS_1 + (1 - x)S_0$$

- Assumes  $m/d = 1$  and  $x=0$

# Convex Routing

- Since  $m/d=1$  and  $x=0$ , then  $S = xKI^{m/d} + (1-x)KO^{m/d}$  reduces to:

$$S = KO$$

- If  $I = I_1$  and  $O = O_1$  in  $I - O = \frac{dS}{dt} \approx \frac{\Delta S}{\Delta t}$  and substituting for  $S$  gives

$$I_1 - O_1 = \frac{K(O_2 - O_1)}{\Delta t}$$

- Rearranging and solving for the unknown  $O_2$  gives  $O_2 = I_1 \frac{\Delta t}{K} - O_1 \frac{\Delta t}{K} + O_1$  or  $O_2 = I_1 \frac{\Delta t}{K} + O_1(1 - \frac{\Delta t}{K})$

# Convex Routing

- Suppose  $C = \frac{\Delta t}{K}$  in  $O_2 = I_1 \frac{\Delta t}{K} + O_1(1 - \frac{\Delta t}{K})$ , then

$$O_2 = CI_1 + O_1(1 - C)$$

- For analysis and synthesis, the upstream hydrograph is known in both cases.
- For analysis, the downstream hydrograph is also known, but the routing coefficient  $C$  is not known.

# Example

- Suppose the routing coefficient of a stream reach is 0.3 and  $I_t = 25$  and  $O_t = 13$  for  $t=0$  and the next  $I_t$  values are 28, 33, and 41, compute the next  $O_t$  value .
- Solution: The routing equation is

$$O_{t+\Delta t} = 0.3I_t + 0.7O_t$$

I	O
25	13
28	$17=0.3(25)+0.7(13)$
33	$20=0.3(28)+0.7(17)$
41	$24=0.3(33)+0.7(20)$

- Assigned task: 4 methods for estimating C (Mc Cuen)

# Wedge and Prism Storage

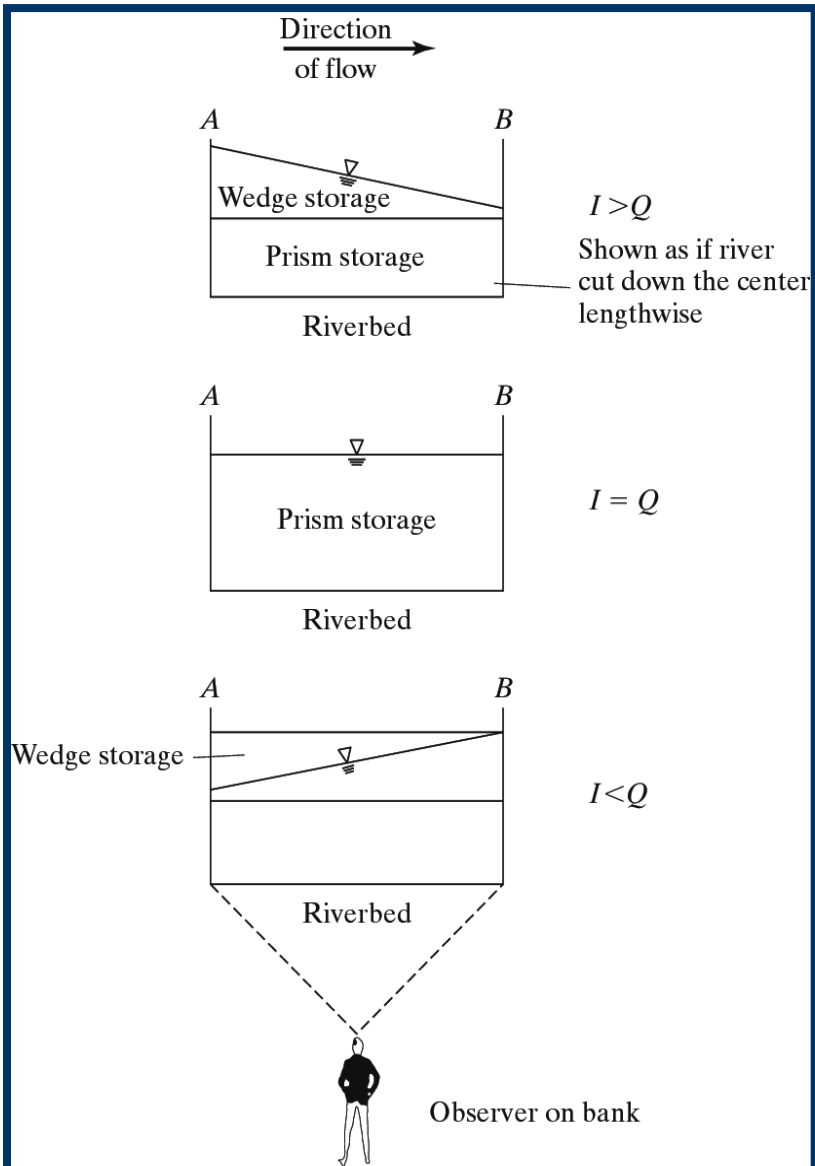


Figure 4.3

Prism and wedge storage concepts.

- **Positive wedge**  $I > Q$
- **Maximum S** when  $I = Q$
- **Negative wedge**  $I < Q$

# Muskingum Method

$$S_p = K O$$

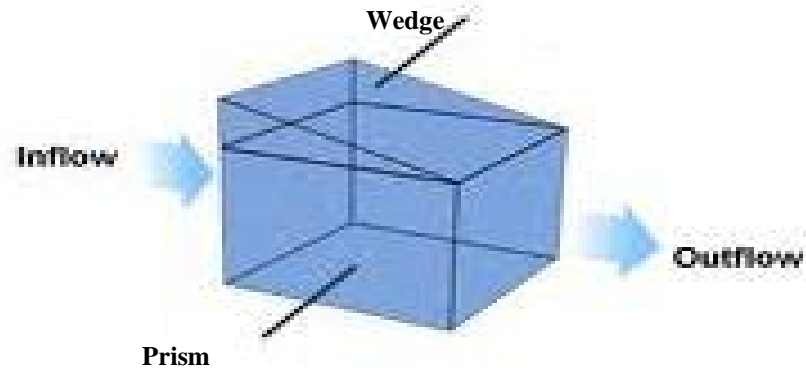
Prism Storage

$$S_w = K(I - O)X$$

Wedge Storage

$$S = K[XI + (1-X)O]$$

Combined



# Hydrologic river routing (Muskingum Method)

## Wedge storage in reach

$$S_{\text{Prism}} = KQ$$

$$S_{\text{Wedge}} = KX(I - Q)$$

**K** = travel time of peak through the reach

**X** = weight on inflow versus outflow ( $0 \leq X \leq 0.5$ )

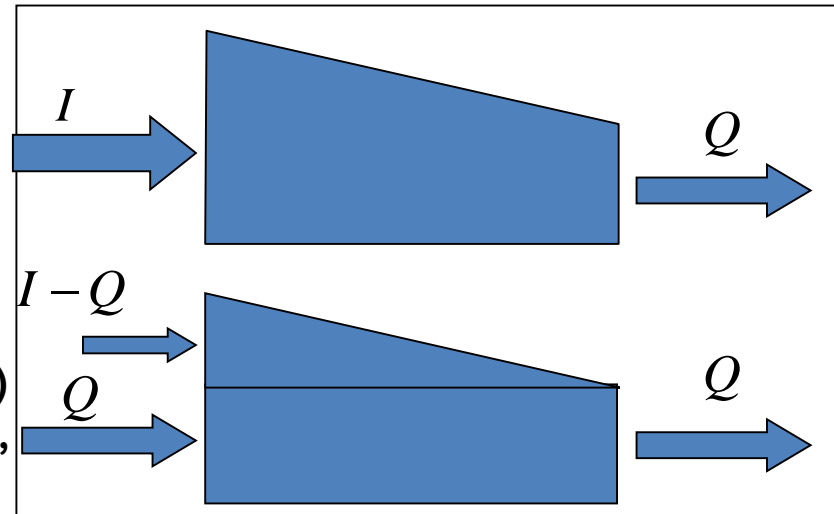
**X = 0** → Reservoir, storage depends on outflow, no wedge

**X = 0.0 - 0.3** → Natural stream

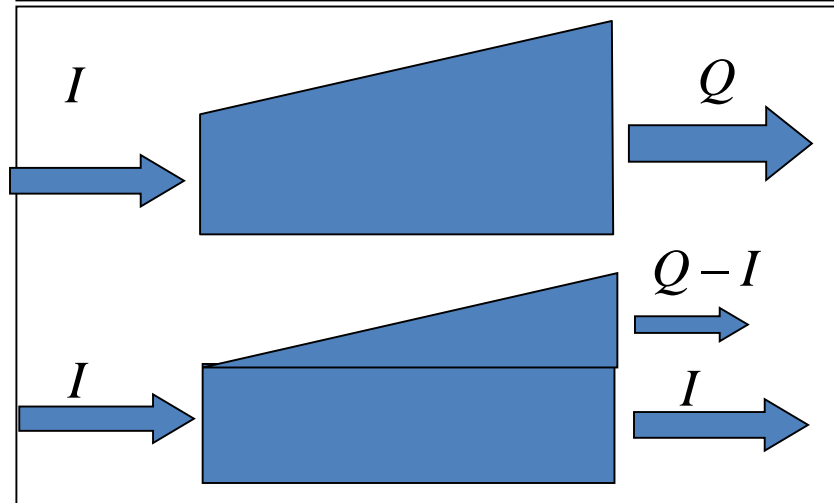
$$S = KQ + KX(I - Q)$$

$$S = K[XI + (1 - X)Q]$$

Advancing  
Flood  
Wave  
 $I > Q$



Receding  
Flood  
Wave  
 $Q > I$





# Muskingum Method for Hydrologic River flood Routing

- Based  $\frac{I_1 + I_2}{2} - \frac{(O_1 + O_2)}{2} = \frac{S_2 - S_1}{\Delta t}$  and the

storage function  $S = xKI^{m/d} + (1 - x)KO^{m/d}$

- Let  $m/d=1$  and  $k=b/a$ , then we have

$$S = K[xI + (1 - x)O]$$

- Substituting into the routing equation gives

$$\frac{I_1 + I_2}{2} - \frac{O_1 + O_2}{2} = \frac{K[xI_1 + (1 - x)O_1] - K[xI_2 + (1 - x)O_2]}{\Delta t}$$

# Muskingum Method

- Solving 
$$\frac{I_1 + I_2}{2} - \frac{O_1 + O_2}{2} = \frac{K[xI_1 + (1-x)O_1] - K[xI_2 + (1-x)O_2]}{\Delta t}$$

and rearranging yields

$$O_2 = C_o I_2 + C_1 I_1 + C_2 O_1$$

Where  $C_o = \frac{-KX + 0.5\Delta t}{K - KX + 0.5\Delta t}$ ,  $C_1 = \frac{KX + 0.5\Delta t}{K - KX + 0.5\Delta t}$  and  $C_2 = 1 - C_o - C_1$

Note: K and  $\Delta t$  must have the same unit and initial estimate of  $O_1$  must be specified as well as K and  $\Delta t$ .

# Muskingum Routing Equation

$$O_2 = C_0 I_2 + C_1 I_1 + C_2 O_1$$

$$O_{t+1} = C_0 I_{t+1} + C_1 I_t + C_2 O_t$$

*where  $C$ 's are functions of  $x$ ,  $K$ ,  $\Delta t$  and sum to 1.0*

# Muskingum Equations

*where*

$$C_0 = (-Kx + 0.5\Delta t) / D$$

$$C_1 = (Kx + 0.5\Delta t) / D$$

$$C_2 = (K - Kx - 0.5\Delta t) / D$$

$$D = (K - Kx + 0.5\Delta t)$$

*Repeat for  $O_3$ ,  $O_4$ ,  $O_5$  and so on.*

# Estimating K

- K is estimated to be the travel time through the reach.
- This may pose somewhat of a difficulty, as the travel time will obviously change with flow.
- The question may arise as to whether the travel time should be estimated using the average flow, the peak flow, or some other flow.
- The travel time may be estimated using the kinematic travel time or a travel time based on Manning's equation.

# Estimating X

- The value of X must be between 0.0 and 0.5.
- The parameter X may be thought of as a weighting coefficient for inflow and outflow.
- As inflow becomes less important, the value of X decreases.
- The lower limit of X is 0.0 and this would be indicative of a situation where inflow, I, has little or no effect on the storage.
- A reservoir is an example of this situation and it should be noted that attenuation would be the dominant process compared to translation.
- Values of  $X = 0.2$  to  $0.3$  are the most common for natural streams; however, values of  $0.4$  to  $0.5$  may be calibrated for streams with little or no flood plains or storage effects.
- A value of  $X = 0.5$  would represent equal weighting between inflow and outflow and would produce translation with little or no attenuation.

# Estimating Muskingum Parameters, K and x

## Graphical Method:

- Referring to the Muskingum Model, find X such that the plot of  $XI_t + (1-X)O_t$  ( $\text{m}^3/\text{s}$ ) vs  $S_t$  ( $\text{m}^3/\text{s.h}$ ) behaves almost nearly as a single value curve. Assume value of x lies between 0 and 0.3.
- The corresponding slope is K.

# Muskingum Routing Procedure

- Given (knowns):  $O_1; I_1, I_2, \dots; \Delta t; K; X$
- Find (unknowns):  $O_2, O_3, O_4, \dots$
- Procedure:
  - (a) Calculate  $C_o, C_1$ , and  $C_2$
  - (b) Apply  $O_{t+1} = C_o I_{t+1} + C_1 I_t + C_2 O_t$  starting from  $t=1, 2, \dots$  recursively.



# Muskingum Notes :

- The method assumes a single stage-discharge relationship.
- In other words, for any given discharge,  $Q$ , there can be only one stage height.
- This assumption may not be entirely valid for certain flow situations.
- For instance, the friction slope on the rising side of a hydrograph for a given flow,  $Q$ , may be quite different than for the recession side of the hydrograph for the same given flow,  $Q$ .
- This causes an effect known as hysteresis, which can introduce errors into the storage assumptions of this method.

# Muskingum Example Problem

- A portion of the inflow hydrograph to a reach of channel is given below. If the travel time is  $K=1$  unit and the weighting factor is  $X=0.30$ , then find the outflow from the reach for the period shown below:

Time	Inflow	$C_0 I_2$	$C_1 I_1$	$C_2 O_1$	Outflow
0	3				3
1	5				
2	10				
3	8				
4	6				
5	5				

# Muskingum Example Problem

- The first step is to determine the coefficients in this problem.
- The calculations for each of the coefficients is given below:

$$C_0 = - \frac{Kx - 0.5\Delta t}{K - Kx + 0.5\Delta t}$$

$$C_0 = - ((1*0.30) - (0.5*1)) / ((1-(1*0.30) + (0.5*1)) = 0.167$$

$$C_1 = \frac{Kx + 0.5\Delta t}{K - Kx + 0.5\Delta t}$$

$$C_1 = ((1*0.30) + (0.5*1)) / ((1-(1*0.30) + (0.5*1)) = 0.667$$

# Muskingum Example Problem

$$C_2 = \frac{K - Kx - 0.5\Delta t}{K - Kx + 0.5\Delta t}$$

$$C_2 = (1 - (1 * 0.30) - (0.5 * 1)) / ((1 - (1 * 0.30) + (0.5 * 1)) = 0.167$$

Therefore the coefficients in this problem are:

- $C_0 = 0.167$
- $C_1 = 0.667$
- $C_2 = 0.167$

# Muskingum Example Problem

- The three columns now can be calculated.
- $C_0 I_2 = 0.167 * 5 = 0.835$
- $C_1 I_1 = 0.667 * 3 = 2.00$
- $C_2 O_1 = 0.167 * 3 = 0.501$

Time	Inflow	$C_0 I_2$	$C_1 I_1$	$C_2 O_1$	Outflow
0	3	0.835	2.00	0.501	3
1	5				
2	10				
3	8				
4	6				
5	5				

# Muskingum Example Problem

- Next the three columns are added to determine the outflow at time equal 1 hour.

- $0.835 + 2.00 + 0.501 = 3.34$

Time	Inflow	$C_0 I_2$	$C_1 I_1$	$C_2 O_1$	Outflow
0	3	0.835	2.00	0.501	3
1	5				3.34
2	10				
3	8				
4	6				
5	5				

# Muskingum Example Problem

- This can be repeated until the table is complete and the outflow at each time step is known.

Time	Inflow	$C_0 I_2$	$C_1 I_1$	$C_2 O_1$	Outflow
0	3	0.835	2.00	0.501	3
1	5	1.67	3.34	0.557	3.34
2	10	1.34	6.67	0.93	5.57
3	8	1.00	5.34	1.49	8.94
4	6	0.835	4.00	1.31	7.83
5	5		3.34	1.03	6.14

End of lecture 6